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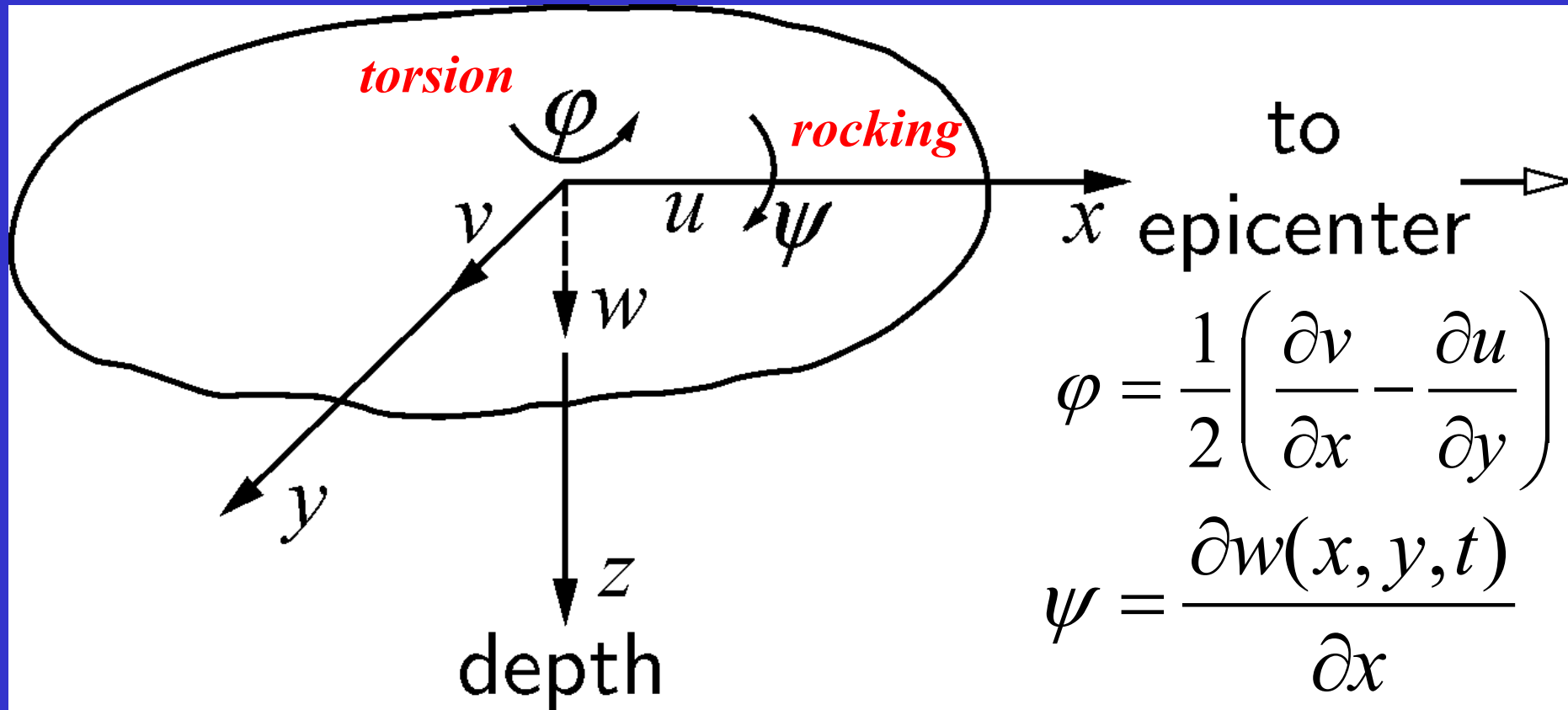
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**Stochastic modeling  
of rocking seismic ground motion  
and respective structural load modeling**  
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# A system of principal<sup>1</sup> axes on the ground surface



components  $u(t)$ ,  $v(t)$ ,  $w(t)$  are uncorrelated

$\varphi$  – horizontal rotation about  $z$  axis (torsion)

$\psi$  – vertical rotation about  $y$  axis (rocking)

<sup>1</sup> Penzien & Watabe Characteristics of 3-D earthquake ground motion, *Earthquake Eng. & Structural Dynamics*, **3**, 1975

# Stochastic model of seismic ground motion

Classic Fourier-Stieltjes representation of stationary random processes:

$$\ddot{u}(t) = \int_{-\infty}^{\infty} e^{i\omega t} d\hat{u}(\omega)$$

the dashed symbol  $\hat{u}(\omega)$  it is random function in frequency domain with orthogonal increments:

$$\langle d\hat{u}(\omega_1) d\hat{u}^*(\omega_2) \rangle = \begin{cases} \langle |d\hat{u}(\omega)|^2 \rangle = S_{\ddot{u}}(\omega) d\omega & \text{for } \omega_1 = \omega_2 = \omega \\ 0 & \text{for } \omega_1 \neq \omega_2 \end{cases}$$

A typical model of ground acceleration it is the Kanai-Tajimi spectrum :

$$S(\omega) = \frac{\omega_g^4 + (2\xi_g \omega_g \omega)^2}{(\omega_g^2 - \omega^2)^2 + (2\xi_g \omega_g \omega)^2} S_0$$

in which the  $\omega_g$  and  $\xi_g$  are parameters representing local site conditions:

# Method of evolutionary spectrum

In 1967 Priestley proposed an evolutionary representation for non-stationary random processes in following form

$$\ddot{u}(t) = \int_{-\infty}^{\infty} A_X(t, \omega) e^{i\omega t} d\hat{u}(\omega)$$

It is the modulating function  $A_X(t, \omega)$  which controls the time-dependence (non-stationarity) of the acceleration process

the dashed symbol  $\hat{u}(\omega)$  it is, as previously, random function in frequency domain with orthogonal increments:

$$\langle d\hat{u}(\omega_1) d\hat{u}^*(\omega_2) \rangle = \begin{cases} \langle |d\hat{u}(\omega)|^2 \rangle = S_{\ddot{u}}(\omega) d\omega & \text{for } \omega_1 = \omega_2 = \omega \\ 0 & \text{for } \omega_1 \neq \omega_2 \end{cases}$$

# Evolutionary Kanai-Tajimi earthquake model

In **1987 Lin & Yong** proposed a generalization of the familiar Kanai-Tajimi model of seismic ground motion to account for non-stationarity of the excitation process

The evolutionary spectral density as proposed by Lin & Yong takes form:

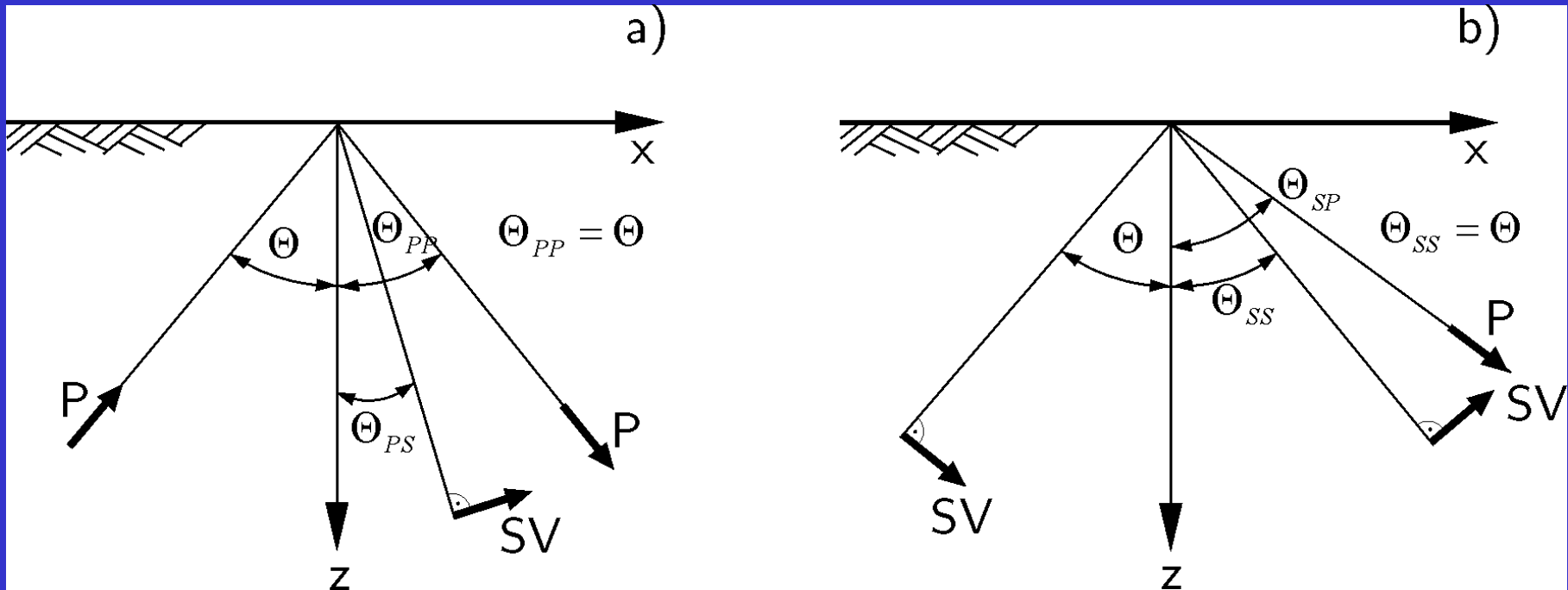
$$S_{\ddot{u}}(t, \omega) = |M(t, \omega)|^2 S_0$$

$$\text{with } M(t, \omega) = \int_0^t h(\tau) \sqrt{\lambda(t-\tau)} e^{i\omega\tau} d\tau$$

$$h(t) = \omega_g \exp[-\xi_g^2 \omega_g t] \left\{ \frac{1-2\xi_g^2}{\sqrt{1-\xi_g^2}} \sin(\omega_{gd}t) + 2\xi_g \cos(\omega_{gd}t) \right\}$$

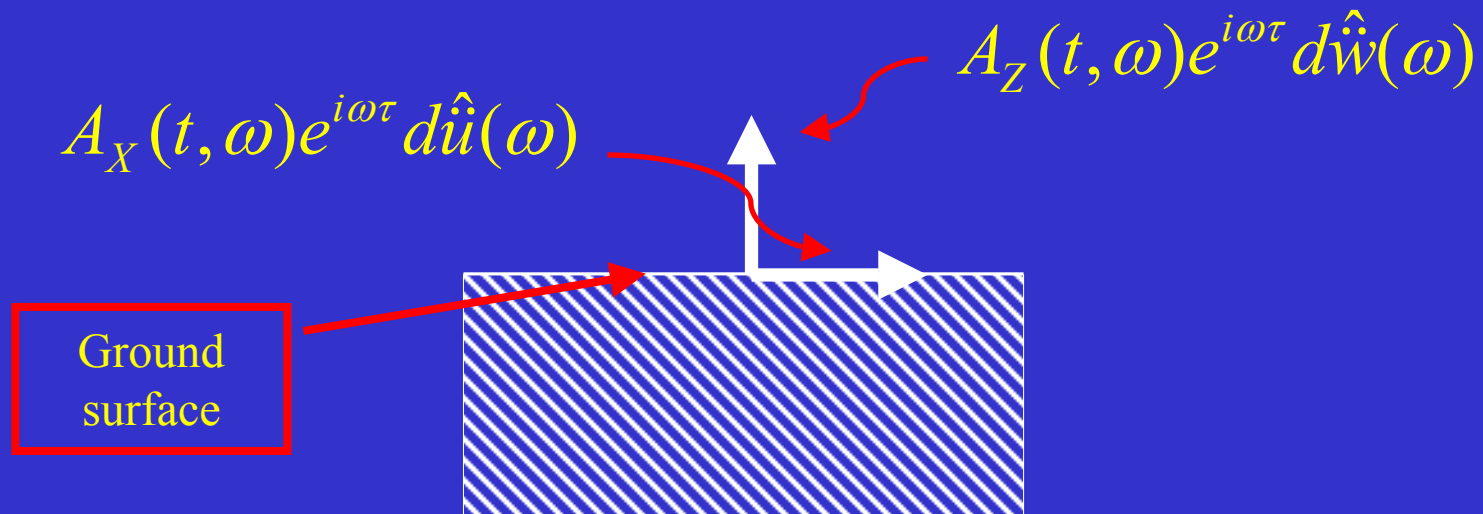
$$\omega_{gd} = \omega_g \sqrt{1-\xi_g^2}$$

# Decomposition of body waves



**Assumption:** the same incidence angle  $\theta$  for both  $P$  &  $SV$  waves

Soil not stratified



Horizontal excitations as contribution of  
body P and SV waves

$$d\ddot{u} = d\ddot{u}_P + d\ddot{u}_S$$

Vertical excitations as contribution of  
body P and SV waves

$$d\ddot{w} = d\ddot{w}_P + d\ddot{w}_S$$

frequency  
band:

$$(\omega; \omega + d\omega)$$

## Substituting the Priestley's spectral representation

$$\left\{ \begin{array}{l} A_X(t, \omega) e^{i\omega\tau} d\hat{u}(\omega) = U_p A_P(t, \omega) e^{i\omega\tau} d\hat{\Phi}_p(\omega) \\ \quad + U_s A_S(t, \omega) e^{i\omega\tau} d\hat{\Phi}_s(\omega) \\ A_Z(t, \omega) e^{i\omega\tau} d\hat{w}(\omega) = W_p A_P(t, \omega) e^{i\omega\tau} d\hat{\Phi}_p(\omega) \\ \quad + W_s A_S(t, \omega) e^{i\omega\tau} d\hat{\Phi}_s(\omega) \end{array} \right.$$

$U_p, U_s$  – horizontal participation coefficients

$W_p, W_s$  – vertical participation coefficients

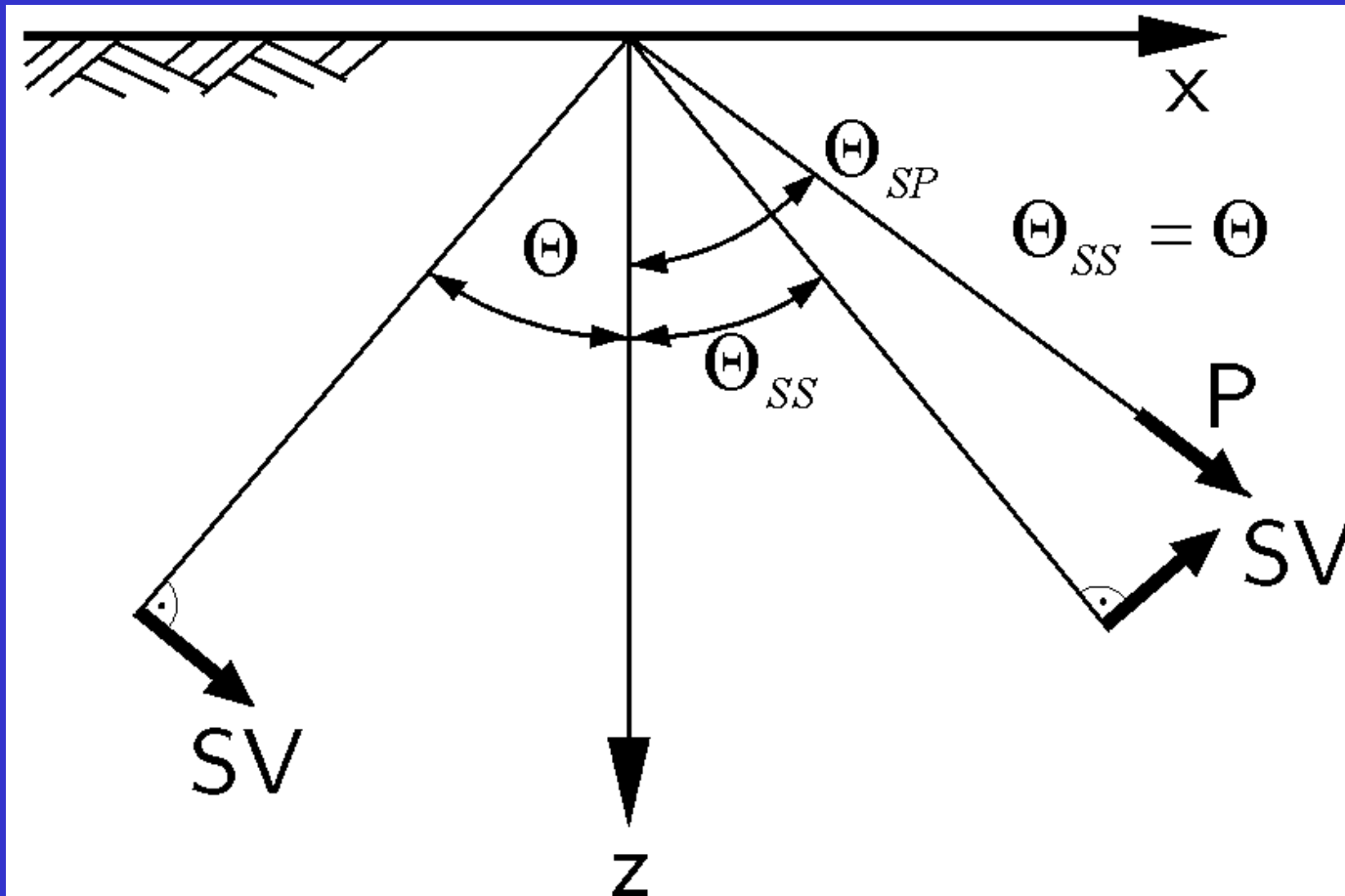




Coefficients  $U_s$  &  $W_s$

$$U_s = (1 + S_s) \cos(\Theta) + S_p \sin(\Theta_{sp})$$

$$W_s = (1 - S_s) \sin(\Theta) - S_p \cos(\Theta_{sp})$$



The coefficients  $P_p$ ,  $P_s$  &  $S_p$ ,  $S_s$  are reflection coefficients of body waves at the free surface

$$P_p = \frac{-\left(\frac{1}{c_s^2} - 2p^2\right)^2 + 4p^2 \frac{\cos(\Theta)}{c_p} \frac{\cos(\Theta_{ps})}{c_s}}{\left(\frac{1}{c_s^2} - 2p^2\right)^2 + 4p^2 \frac{\cos(\Theta)}{c_p} \frac{\cos(\Theta_{ps})}{c_s}}$$

$$S_p = \frac{4 \frac{c_p}{c_s} p \frac{\cos(\Theta)}{c_s} \left(\frac{1}{c_s^2} - 2p^2\right)}{\left(\frac{1}{c_s^2} - 2p^2\right)^2 + 4p^2 \frac{\cos(\Theta_{sp})}{c_p} \frac{\cos(\Theta)}{c_s}}$$

$$P_s = \frac{4 \frac{c_p}{c_s} p \frac{\cos(\Theta)}{c_p} \left(\frac{1}{c_s^2} - 2p^2\right)}{\left(\frac{1}{c_s^2} - 2p^2\right)^2 + 4p^2 \frac{\cos(\Theta)}{c_p} \frac{\cos(\Theta_{ps})}{c_s}}$$

$$S_s = \frac{\left(\frac{1}{c_s^2} - 2p^2\right)^2 - 4p^2 \frac{\cos(\Theta_{sp})}{c_p} \frac{\cos(\Theta)}{c_s}}{\left(\frac{1}{c_s^2} - 2p^2\right)^2 + 4p^2 \frac{\cos(\Theta_{sp})}{c_p} \frac{\cos(\Theta)}{c_s}}$$

Achenbach, *Wave propagation in elastic solids*, North-Holland, Amsterdam 1973

Aki, Richards, *Quantitative Seismology*, W.H. Freeman, San Francisco 1980,

$$\sin(\Theta) / \sin(\Theta_{ps}) = c_p / c_s = S$$

$$\sin(\Theta) / \sin(\Theta_{sp}) = c_s / c_p$$

$C_p$  = propagation velocity of P waves

$C_s$  = propagation velocity of S waves

$p = \sin(\Theta) / c_p$  = horizontal slowness of incident P waves (inverse of velocity)

$$\left\{ \begin{array}{l} A_X(t, \omega) e^{i\omega\tau} d\hat{u}(\omega) = U_p A_P(t, \omega) e^{i\omega\tau} d\hat{\Phi}_p(\omega) \\ \quad + U_s A_S(t, \omega) e^{i\omega\tau} d\hat{\Phi}_s(\omega) \\ A_Z(t, \omega) e^{i\omega\tau} d\hat{w}(\omega) = W_p A_P(t, \omega) e^{i\omega\tau} d\hat{\Phi}_p(\omega) \\ \quad + W_s A_S(t, \omega) e^{i\omega\tau} d\hat{\Phi}_s(\omega) \end{array} \right.$$

solving the initial system of equations for  $A_P$  and  $A_S$  gives:

$$\left\{ \begin{array}{l} A_P(t, \omega) e^{i\omega t} d\hat{\Phi}_p(\omega) = \frac{W_s}{D} A_X(t, \omega) e^{i\omega t} d\hat{u}(\omega) \\ \quad - \frac{U_s}{D} A_Z(t, \omega) e^{i\omega t} d\hat{w}(\omega) \\ A_S(t, \omega) e^{i\omega t} d\hat{\Phi}_s(\omega) = \frac{U_p}{D} A_Z(t, \omega) e^{i\omega t} d\hat{w}(\omega) \\ \quad - \frac{W_s}{D} A_X(t, \omega) e^{i\omega t} d\hat{u}(\omega) \end{array} \right.$$

The vertical acceleration in the frequency band  $(\omega; \omega+d\omega)$  is now decomposed onto two wave terms:



$$d\ddot{w}(t, \omega, x) = W_p \exp\left[i\omega\left(t - \frac{x \sin(\Theta)}{c_p}\right)\right] d\hat{\Phi}_p(\omega) + W_s \exp\left[i\omega\left(t - \frac{x \sin(\Theta)}{c_s}\right)\right] d\hat{\Phi}_s(\omega)$$

Differentiating the vertical field with respect to spatial, horizontal coordinate  $x$  gives the rocking component

$$d\ddot{\psi}(t, \omega, x) = \frac{\partial}{\partial x} d\ddot{w}(t, \omega, x) \Big|_{x=0}$$

# Rocking acceleration

$$\begin{aligned}d\ddot{\psi}(t, \omega) = & W_p A_X(t, \omega) \left( -i\omega \frac{\sin(\Theta)}{c_p} \right) e^{i\omega t} \frac{W_s}{D} d\hat{u}(\omega) \\ & - W_p A_Z(t, \omega) \left( -i\omega \frac{\sin(\Theta)}{c_p} \right) e^{i\omega t} \frac{U_s}{D} d\hat{w}(\omega) \\ & + W_s A_Z(t, \omega) \left( -i\omega \frac{\sin(\Theta)}{c_p} \right) e^{i\omega t} \frac{U_p}{D} d\hat{w}(\omega) \\ & - W_s A_X(t, \omega) \left( -i\omega \frac{\sin(\Theta)}{c_s} \right) e^{i\omega t} \frac{W_p}{D} d\hat{u}(\omega)\end{aligned}$$

## Introducing new coefficients

$$W_x = \frac{W_p W_s \sin(\Theta)}{D c_p} - \frac{W_p W_s \sin(\Theta)}{D c_s}$$

$$W_z = \frac{U_p W_s \sin(\Theta)}{D c_s} - \frac{W_p U_s \sin(\Theta)}{D c_p}$$

we have

$$d\ddot{\psi}(t, \omega) = W_x(-i\omega) A_X(t, \omega) e^{i\omega t} d\hat{u}(\omega) + W_z(-i\omega) A_Z(t, \omega) e^{i\omega t} d\hat{w}(\omega)$$

After integration in the whole frequency domain we have rocking acceleration:

$$\ddot{\psi}(t) = \int_{-\infty}^{\infty} W_x(-i\omega) A_X(t, \omega) e^{i\omega t} d\hat{u}(\omega) + \int_{-\infty}^{\infty} W_z(-i\omega) A_Z(t, \omega) e^{i\omega t} d\hat{w}(\omega)$$

This equation can be used to obtain any probabilistic characteristics of rotation

For example the mean square rotational acceleration

$$\begin{aligned} \sigma_{\ddot{\psi}}^2(t) = & \int_{-\infty}^{\infty} |W_x|^2 |A_X(t, \omega)|^2 \omega^2 S_{\ddot{u}}(\omega) d\omega + 2 \int_{-\infty}^{\infty} W_x W_z^* A_X(t, \omega) A_Z^*(t, \omega) \omega^2 S_{\ddot{u}\ddot{w}}(\omega) d\omega \\ & + \int_{-\infty}^{\infty} |W_z|^2 |A_Z(t, \omega)|^2 \omega^2 S_{\ddot{w}}(\omega) d\omega \end{aligned}$$

Introducing the Priestely's formulae for evolutionary spectrum

$$\sigma_{\ddot{\psi}}^2(t) = \int_{-\infty}^{\infty} |W_x|^2 \omega^2 S_{\ddot{u}}(t, \omega) d\omega + 2 \int_{-\infty}^{\infty} W_x W_z^* \omega^2 S_{\ddot{u}\ddot{w}}(t, \omega) d\omega + \int_{-\infty}^{\infty} |W_z|^2 \omega^2 S_{\ddot{w}}(t, \omega) d\omega$$

The integrand in this equation it is the evolutionary spectrum

$$S_{\ddot{\psi}}(t, \omega) = |W_x|^2 \omega^2 S_{\ddot{u}}(t, \omega) + 2W_x W_z^* \omega^2 S_{\ddot{u}\ddot{w}}(t, \omega) + |W_z|^2 \omega^2 S_{\ddot{w}}(t, \omega)$$

In the stationary case:

$$S_{\ddot{\psi}}(\omega) = |W_x|^2 \omega^2 S_{\ddot{u}}(\omega) + 2W_x W_z^* \omega^2 S_{\ddot{u}\ddot{w}}(\omega) + |W_z|^2 \omega^2 S_{\ddot{w}}(\omega)$$



$$S_{\ddot{\psi}}(\omega) = |W_x|^2 \omega^2 S_{\ddot{u}}(\omega) + 2W_x W_z^* \omega^2 S_{\ddot{u}\ddot{w}}(\omega) + |W_z|^2 \omega^2 S_{\ddot{w}}(\omega)$$

the spectrum of the rotational component is then a function of the first derivative of accelerations and the third derivative of translations

$$S_{\ddot{u}}(\omega) = \omega^2 S_{\ddot{u}}(\omega) = \omega^4 S_{\dot{u}}(\omega) = \omega^6 S_u(\omega)$$

# Example:

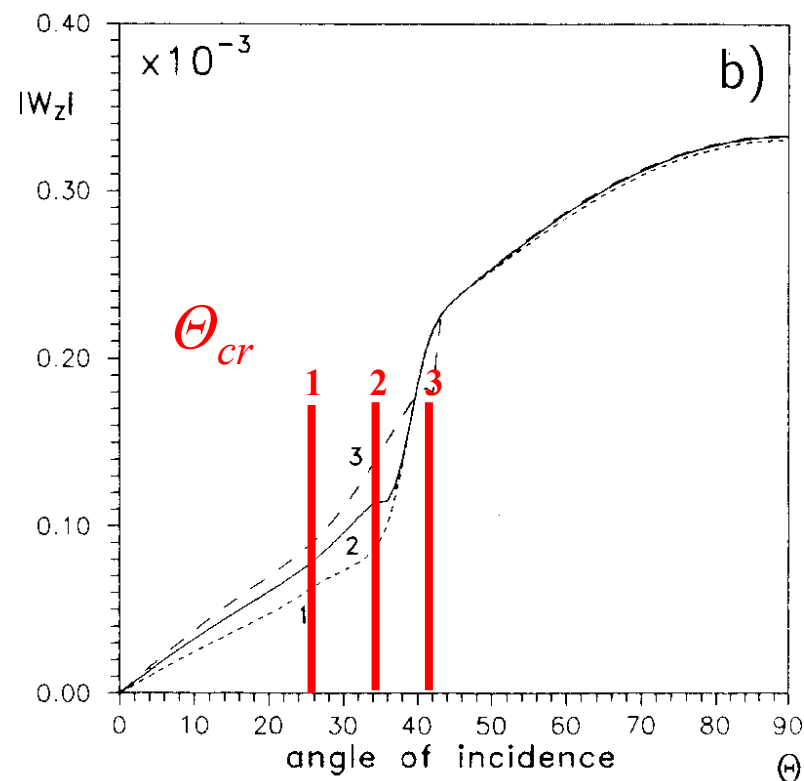
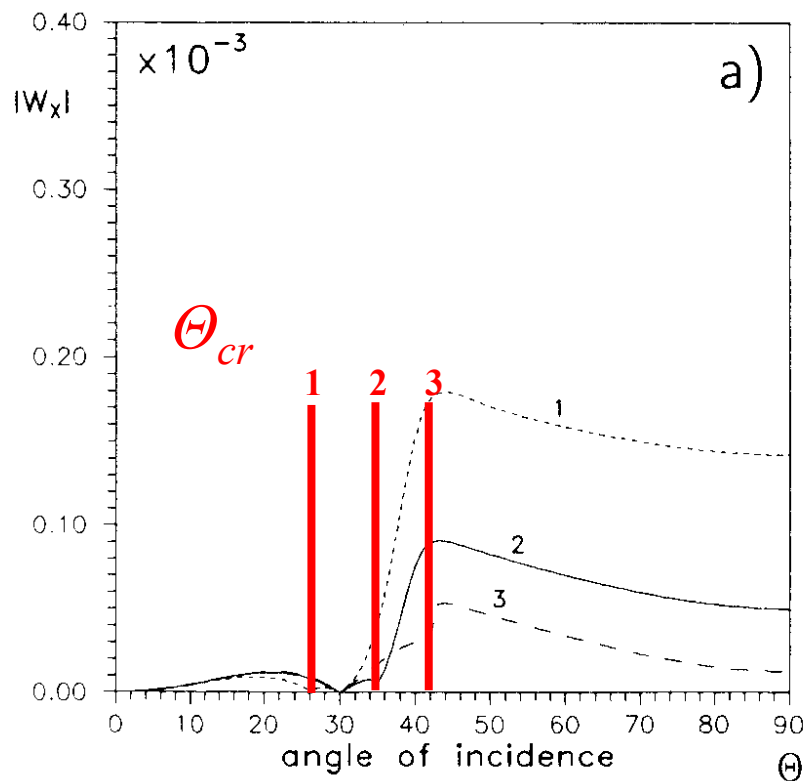
$c_p$ [m/s]	6800	5200	4500
$c_s$ [m/s]	3000	3000	3000
$S=c_p/c_s$	2.27	1.73	1.50
Poisson modulus $\nu$	0.38	0.25	0.10
$\Theta_{cr}$ [deg]	26.14	35.26	41.81

# Modulae

of horizontal  $W_X$  and

vertical  $W_Z$  components vs. incidence angle  $\Theta$  and three values of Poisson coefficient  $\nu$

- 1  $\nu=0.38$   $\Theta_{cr}=26.14^\circ$
- 2  $\nu=0.25$   $\Theta_{cr}=35.26^\circ$
- 3  $\nu=0.10$   $\Theta_{cr}=41.81^\circ$



for  $\Theta < \Theta_{cr}$   $W_X, W_Z \in \mathcal{R}$  and  $W_X < 0, W_Z > 0$   
for  $\Theta > \Theta_{cr}$   $W_X, W_Z \in \mathcal{C}$

## Problem of correlation between horizontal and vertical components

Applying Penzien & Watabe (1975) assumption of no-correlation we may omit the cross spectrum term:

$$S_{\ddot{y}}(t, \omega) = |W_x|^2 \omega^2 S_{\ddot{u}}(t, \omega) + \cancel{2W_x W_z^* \omega^2 S_{\ddot{u}\ddot{w}}(t, \omega)} + |W_z|^2 \omega^2 S_{\ddot{w}}(t, \omega)$$

Assuming also the same spectrum for horizontal & vertical component but with different intensity factors such that:  $S_{\ddot{w}}(t, \omega) = \eta^2 S_{\ddot{u}}(\omega)$

For uncorrelated hor-ver motions we have following formula :

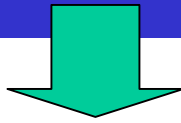
$$S_{\ddot{y}}(t, \omega) = (|W_x|^2 + \eta^2 |W_z|^2) \omega^2 S_{\ddot{u}}(t, \omega)$$

And on the opposite side for total (100%) correlation of hor-ver motions :

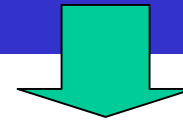
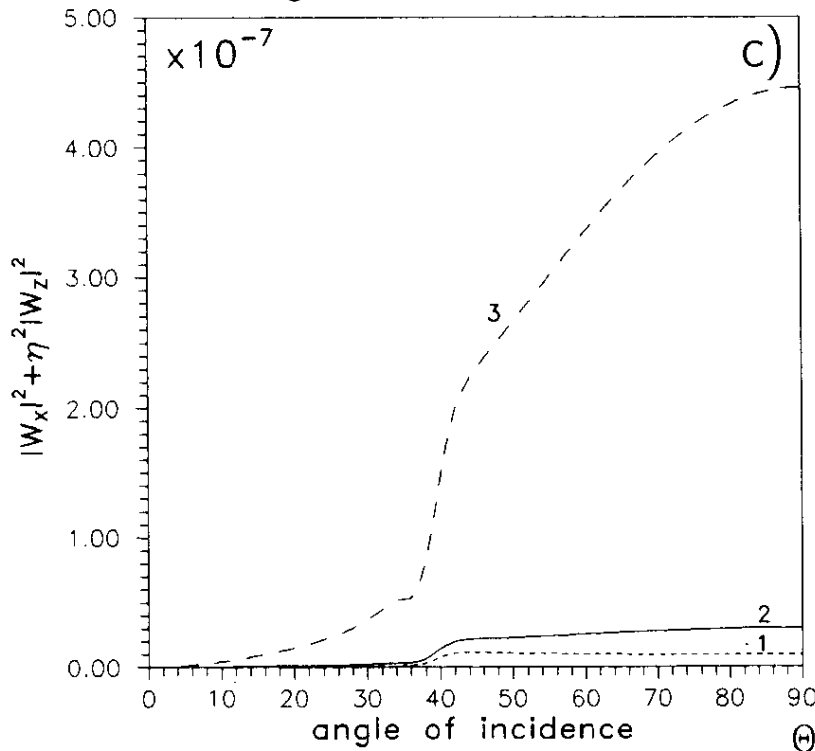
$$S_{\ddot{y}}(t, \omega) = (|W_x|^2 + 2\eta W_x W_z^* + \eta^2 |W_z|^2) \omega^2 S_{\ddot{u}}(t, \omega)$$

Both spectral multipliers vs. incidence angle, for stationary processes  
for three values of  $\eta$ : (1)  $\eta=0.25$ , (2)  $\eta=0.50$ , (3)  $\eta=2.0$

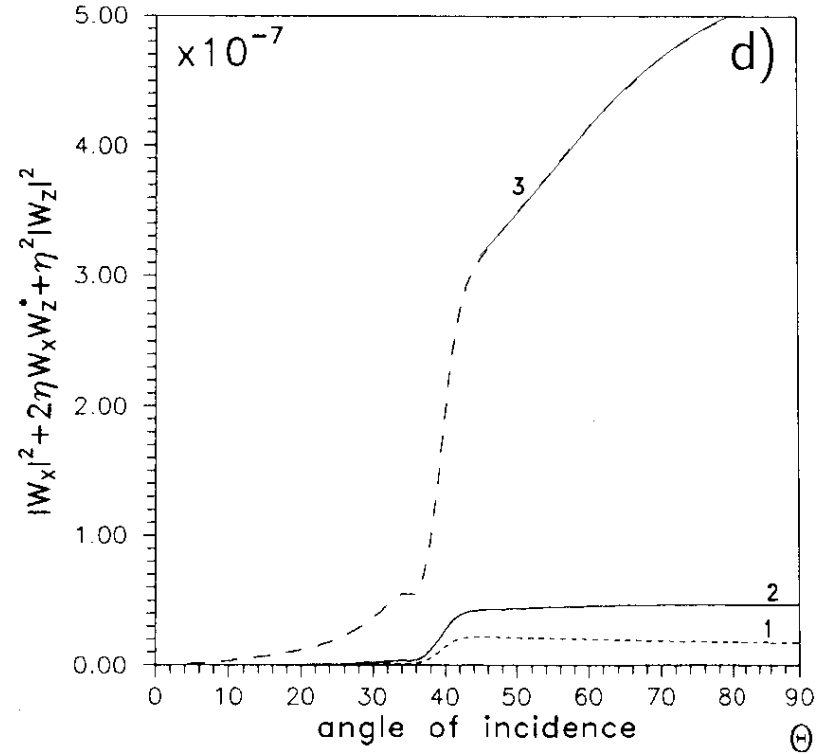
$$S_{\ddot{\psi}}(\omega) = (|W_X|^2 + \eta^2 |W_Z|^2) \omega^2 S_{\ddot{u}}(\omega) \quad S_{\ddot{\psi}}(\omega) = (|W_X|^2 + 2\eta W_X W_Z^* + \eta^2 |W_Z|^2) \omega^2 S_{\ddot{u}}(\omega)$$



0% correlation „hor-ver”



100% correlation „hor-ver”



# translational & rotational spectrum

$$S_{\ddot{u}}(\omega) = \frac{\omega^2}{\left(1 + \frac{\omega}{\omega_c}\right)^2} \exp\left[-\frac{1}{2} \kappa \omega\right] S_0$$

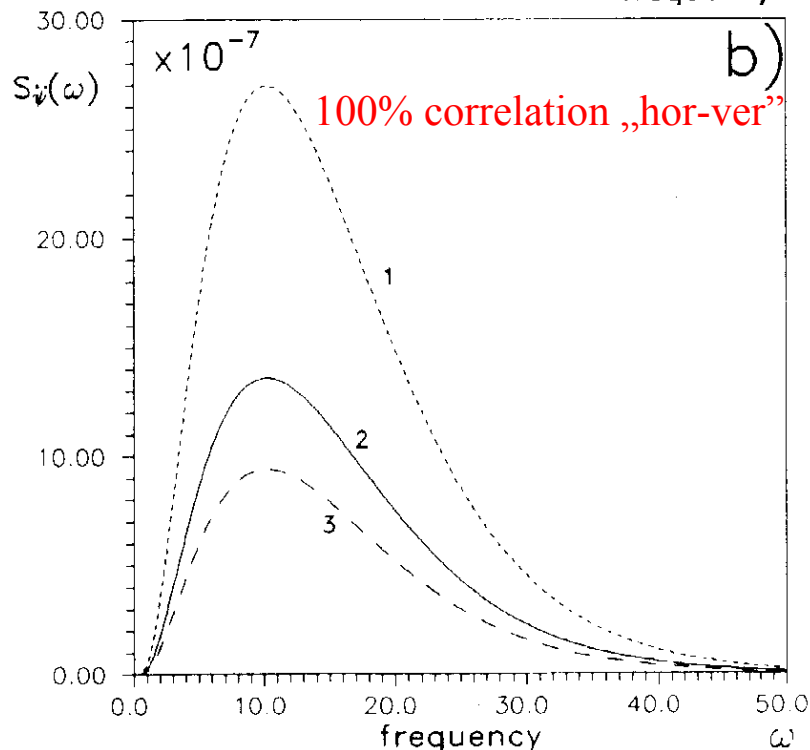
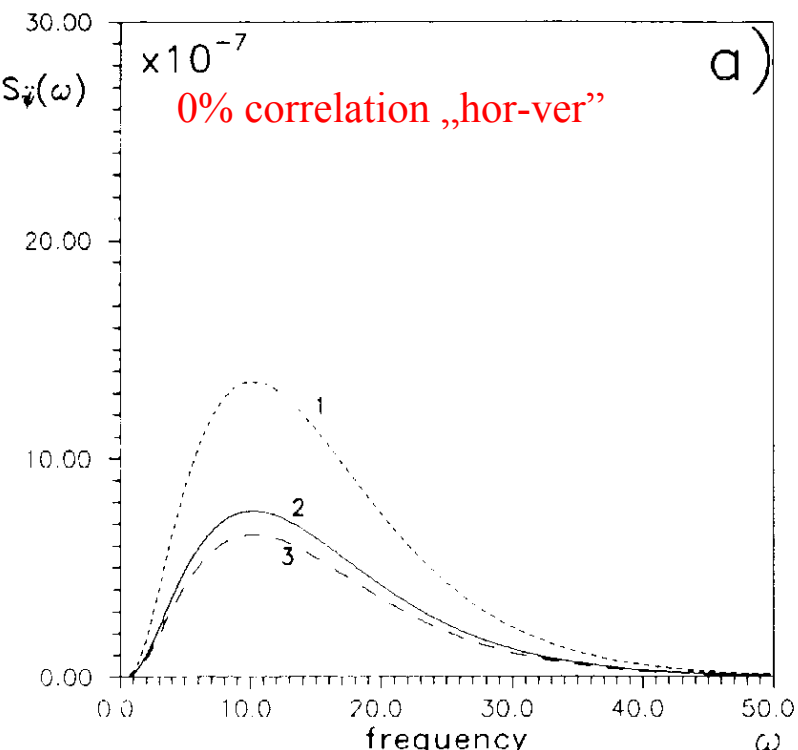
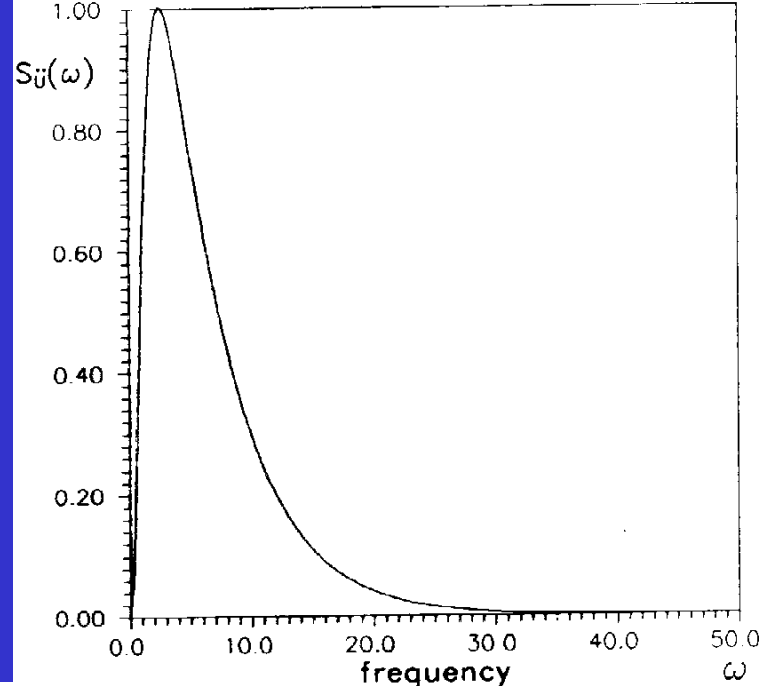
Brune spectrum;  $\omega_c$  = corner frequency

$\eta = 0.5, \Theta = 60^\circ$

1  $\nu = 0.38$

2  $\nu = 0.25$

3  $\nu = 0.10$



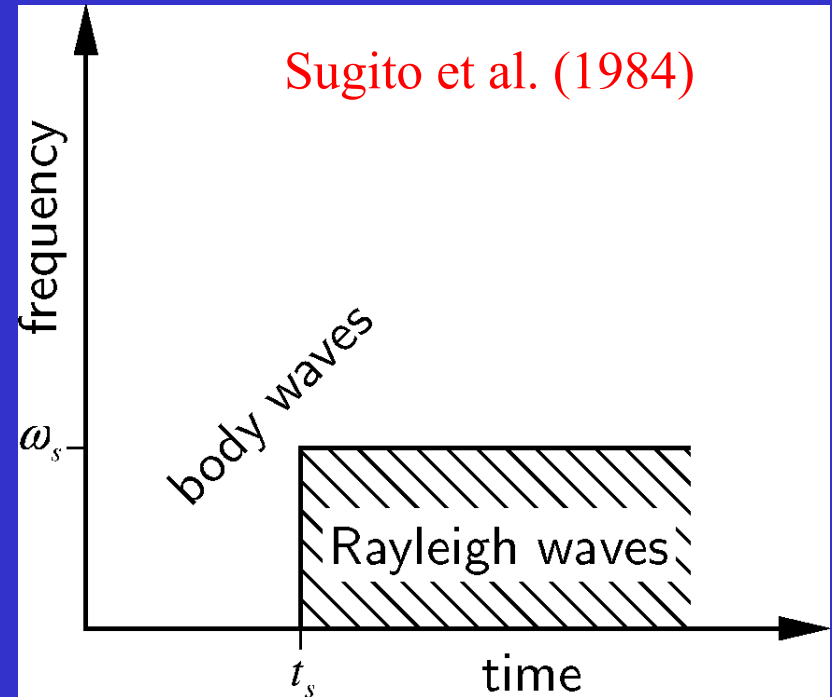
# Surface waves effects

$$\ddot{u}(t) = \ddot{u}_R(t) + \ddot{u}_b(t) \quad - \text{horizontal}$$

$$\ddot{w}(t) = \ddot{w}_R(t) + \ddot{w}_b(t) \quad - \text{vertical}$$

incremental acceleration of surface waves for  $(\omega; \omega+d\omega)$ :

$$d\ddot{w}_R = A_R(t, \omega) \exp\left[i\omega\left(t - \frac{x}{c_R}\right)\right] d\hat{w}(\omega) \quad c_R = \begin{cases} \text{velocity of} \\ \text{Rayleigh waves} \end{cases}$$



## The rotation due to Rayleigh waves

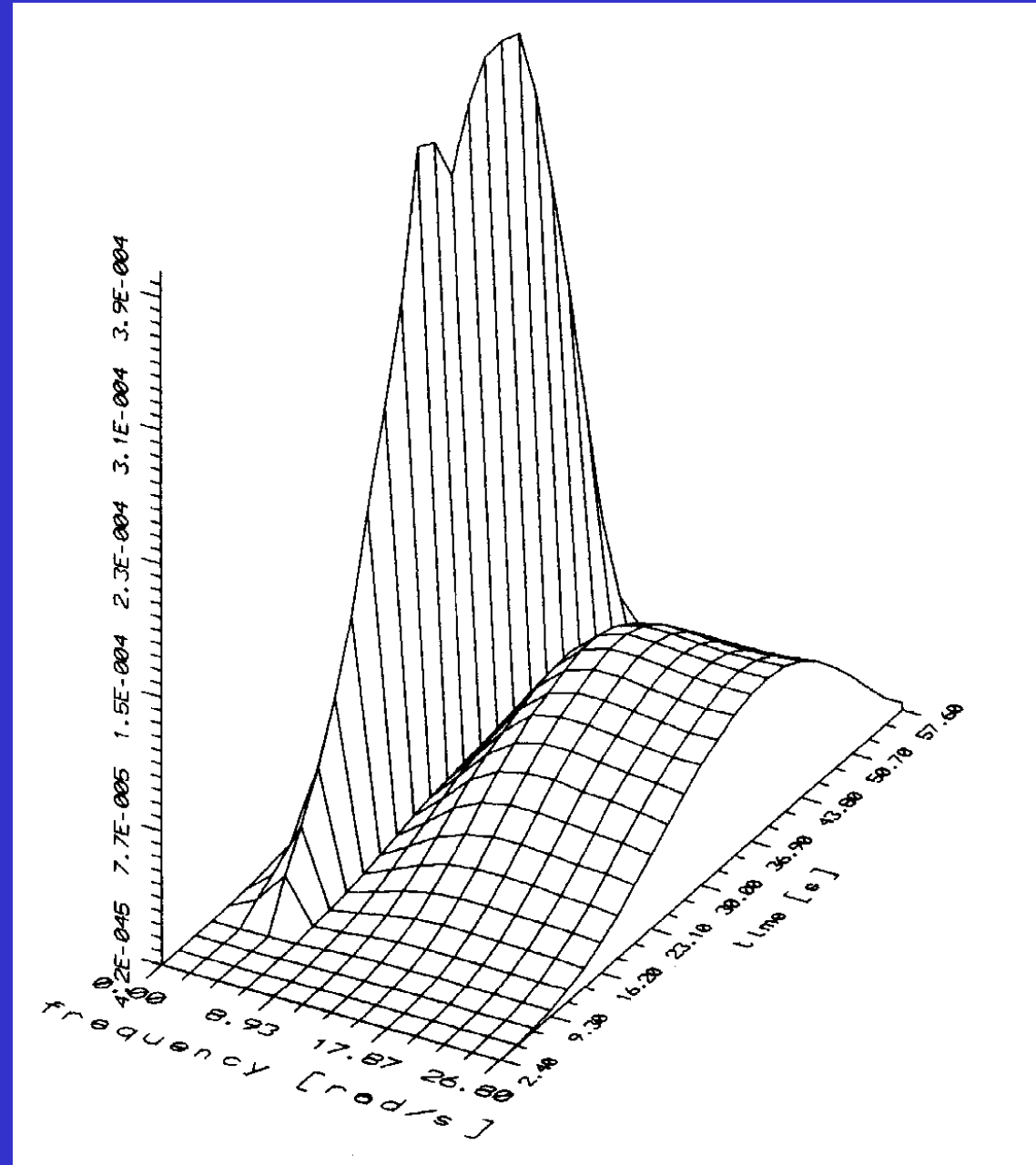
$$d\ddot{\psi}_R = \frac{\partial}{\partial x} d\ddot{w}_R(t, \omega, x)|_{x=0} = A_R(t, \omega) \left( -\frac{i\omega}{c_R} \right) e^{i\omega t} d\hat{w}(\omega)$$

Introducing  $W_R = 1/c_R$  we obtain final formula  
for mean square rocking from both body & surface waves

$$\sigma_{\ddot{\psi}}^2(t) = \begin{cases} 2 \int_0^{\infty} [W_X^2 \omega^2 S_{\ddot{u}}(t, \omega) + 2W_X W_Z^* \omega^2 S_{\ddot{u}\ddot{w}}(t, \omega) + W_Z^2 \omega^2 S_{\ddot{w}}(t, \omega)] d\omega & \text{for } t < t_S \\ 2 \int_0^{\omega_S} W_R^2 \omega^2 S_{\ddot{w}}(t, \omega) d\omega + 2 \int_{\omega_S}^{\infty} [W_X^2 \omega^2 S_{\ddot{u}}(t, \omega) + 2W_X W_Z^* \omega^2 S_{\ddot{u}\ddot{w}}(t, \omega) + W_Z^2 \omega^2 S_{\ddot{w}}(t, \omega)] d\omega & \text{for } t > t_S \end{cases}$$



# Rotational evolutionary Kanai-Tajimi spectrum due to joint effect of body and surface waves:



# Conclusions from the first part

The final formula for the spectral density of the rotation due to body waves:

$$S_{\ddot{\psi}}(t, \omega) = |W_x|^2 \omega^2 S_{\ddot{u}}(t, \omega) + 2W_x W_z^* \omega^2 S_{\ddot{u}\ddot{w}}(t, \omega) + |W_z|^2 \omega^2 S_{\ddot{w}}(t, \omega)$$

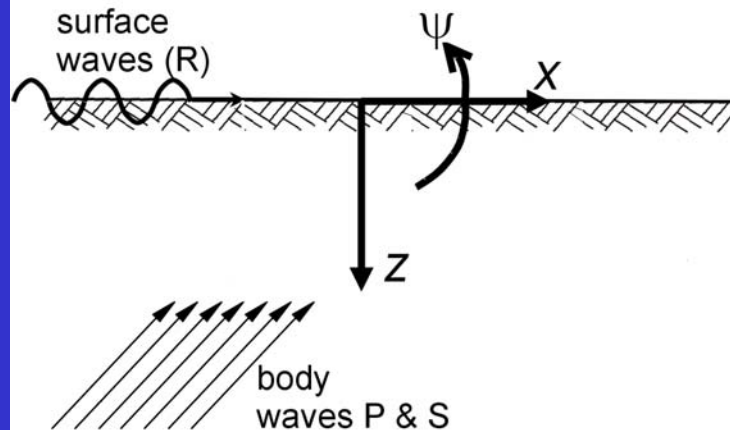
It is a function of the translational spectral densities and their cross-spectrum

$$\left\{ \begin{array}{l} S_{\ddot{u}}(t, \omega) \\ S_{\ddot{w}}(t, \omega) \end{array} \right. \quad S_{\ddot{u}\ddot{w}}(t, \omega)$$

as well as wave and sub-soil parameters:  $W_x, W_z = f(\Theta, c_P, c_S)$

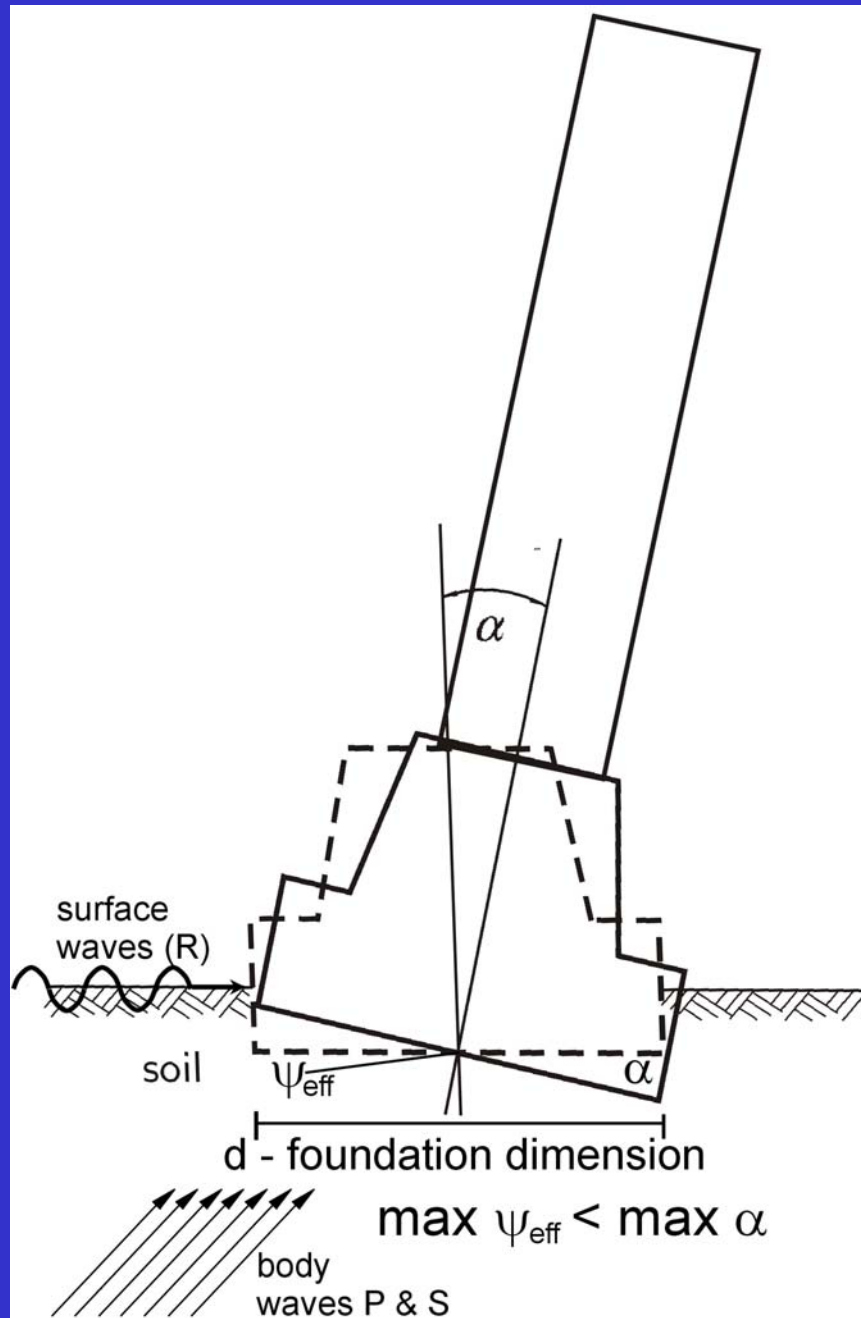
The  $\omega^2$  parameter shifts the rotational spectrum into higher frequencies and in the stationary case introduces direct dependence with the time derivative of the acceleration

# Transforming „point” rotation onto structural load

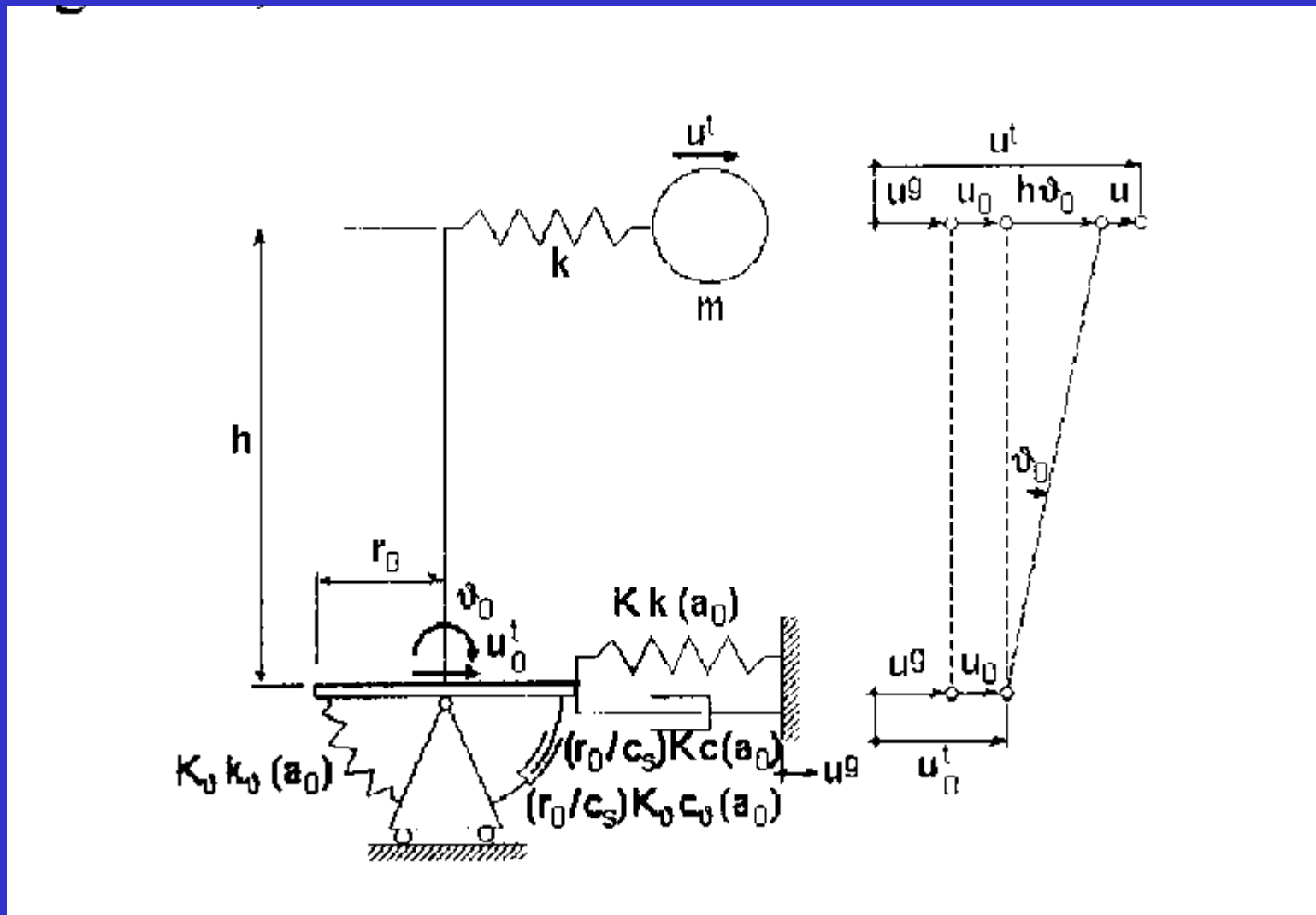




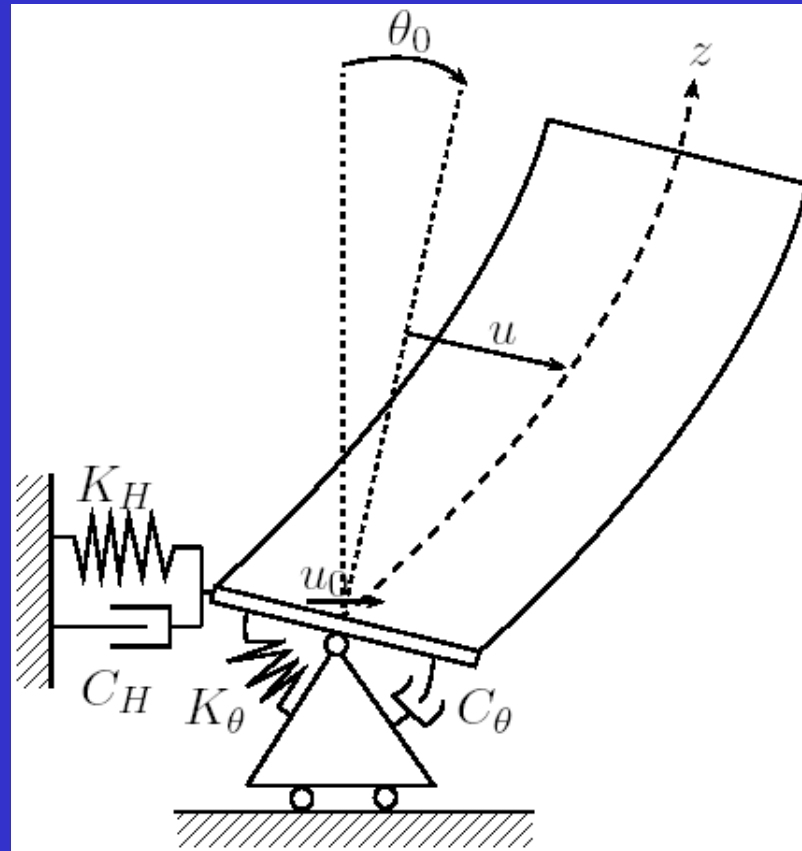
Question: How to calculate  $\psi_{\text{eff}}(t)$  from  $\psi(t)$



# The simplest possible model of a structure under horizontal-rocking excitations



# More advanced model of high-rise structure: A shear beam under horizontal-rocking excitations



# Parameters for simple systems to derive from so called „cone” ground models

Translational  
cone

Rotational  
cone

